

Numerical Integration in Time

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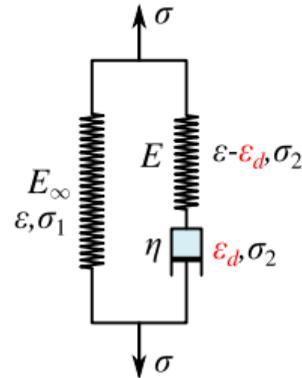
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Motivation

- In nonlinear materials, first order differential equations govern the change of history variables. For example, in viscoelastic material model

$$\sigma = (E + E_\infty)\varepsilon + E\varepsilon_d, \quad \dot{\varepsilon}_d + \frac{\varepsilon_d}{\tau} = \frac{\dot{\varepsilon}}{\tau}, \quad \tau = \eta/E$$

with Young's moduli E, E_∞ (Pa), total ε and viscous ε_d strain, viscosity η (Pa·s), relaxation time τ (s).



©Formula 1

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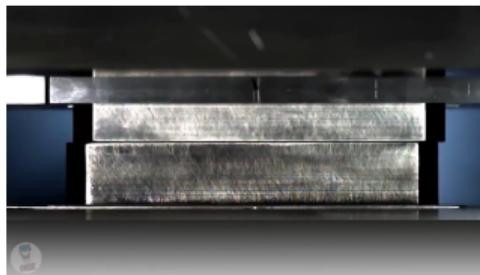
- In non-stationary processes governed by parabolic equations. For example, heat equation

$$\rho c_p \frac{\partial T}{\partial t} = \nabla \cdot (k \nabla T) \quad \Leftrightarrow \quad \boxed{\dot{T} = \alpha \Delta T}$$

ρ - density, c_p specific heat capacity at constant pressure, k thermal conductivity.



Additive manufacturing, ©DMG MORI



Friction welding

- In solid dynamics, hyperbolic PDE:

$$\nabla \cdot \underline{\underline{\sigma}} + \underline{\underline{f}} = \rho \underline{\underline{\ddot{u}}}$$



Lego-car crash simulation in LS-DYNA, ©DYNAMORE

Variable separation

- Search solution in time:

$$\{\underline{\mathbf{X}}, t\} \in \Omega \times (0, T] \rightarrow \underline{u}(\underline{\mathbf{X}}, t)$$

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$$\underline{u}(\underline{\mathbf{X}}, t) = \sum N_i(\underline{\mathbf{X}})\underline{u}_i(t)$$

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- Results in 2nd order in time system of ODE:

$$[M][\ddot{u}] + [C][\dot{u}] + [K][u] = [F](t)$$

with mass matrix $[M] \in \mathbb{R}^{n \times n}$,
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- Or in 1st order in time system of ODE:

$$[C][\dot{T}] + [K][T] = [Q](t)$$

First order differential equations

- Consider a linear first order system of ODE

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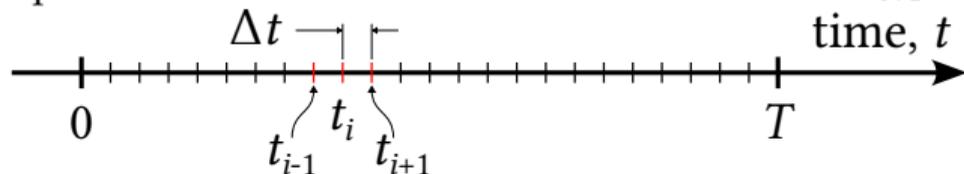
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then $\forall [q_0] \in \mathbb{R}^n$, a unique solution $[q(t)]$ for Cauchy problem exists.

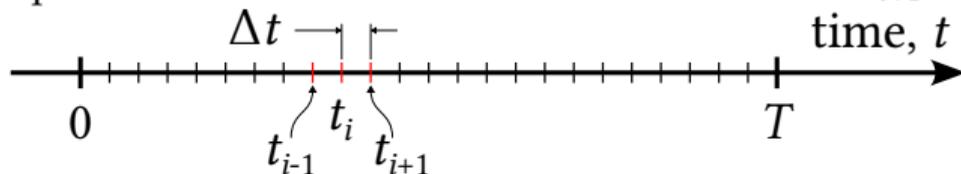
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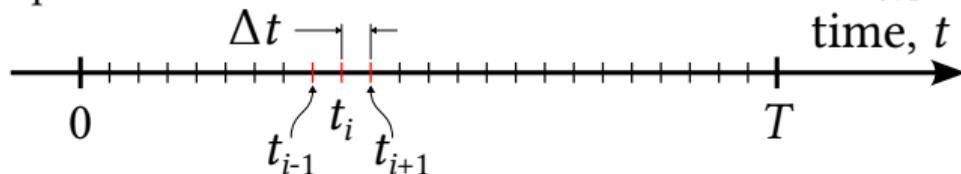
- Taylor expansion:

$$[q(t + \Delta t)] = [q(t)] + [\dot{q}(t)]\Delta t + \frac{1}{2}[\ddot{q}(t)]\Delta t^2 + o(\Delta t^2)$$

with Bachmann-Landau or asymptotic notations: $y = o(x)$ if $y/x \xrightarrow{x \rightarrow 0} 0$

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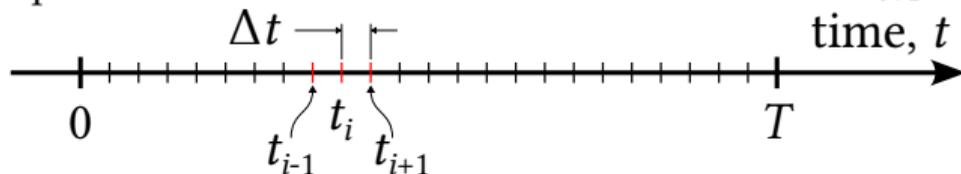
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- So we search discrete values: $[q]_k = [q(t_k)]$
- An integration method is consistent iff

$$\lim_{\Delta t \rightarrow 0} \frac{[q]_{k+1} - [q]_k}{\Delta t} = [\dot{q}(t_k)]$$

Integration in time

- We know that

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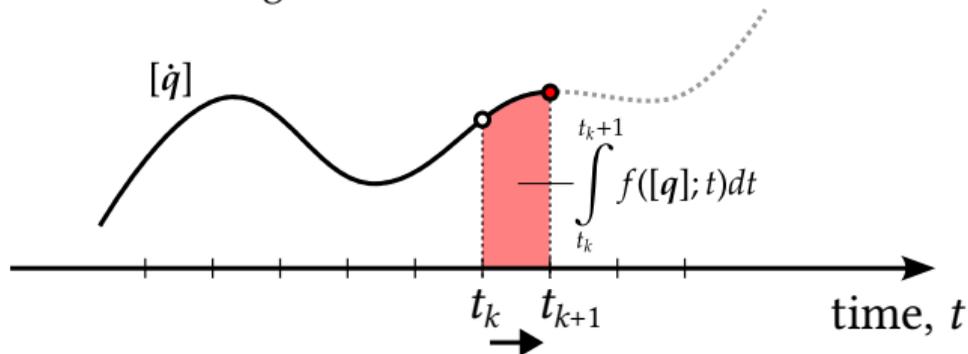
$$[q]_{k+1} = [q]_k + \int_{t_k}^{t_{k+1}} [\dot{q}] dt = [q]_k + \int_{t_k}^{t_{k+1}} f([q]; t) dt$$

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- Why not to use known integration methods?

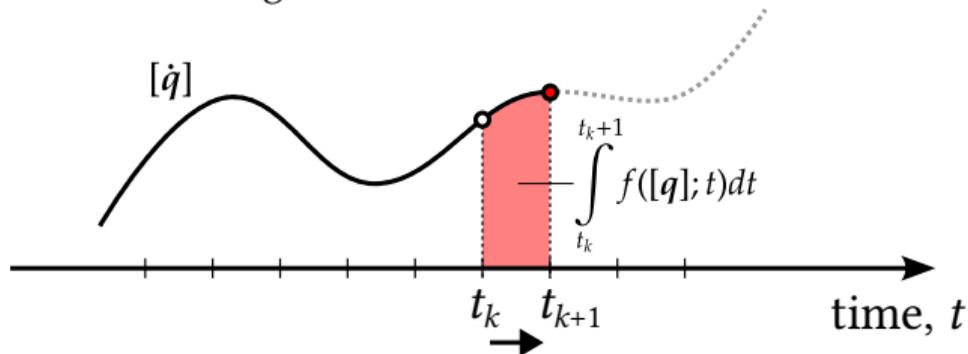


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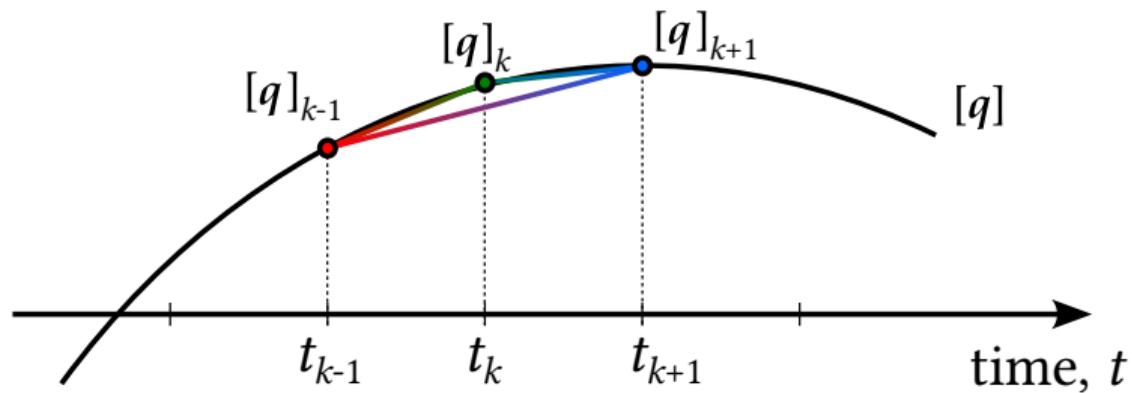
- Why not to use known integration methods?



- Because the value of the integrand is unknown

$$\int_{t_k}^{t_{k+1}} f([q]; t) dt = ?$$

Finite difference



- Consider left and right Taylor expansions:

$$[q(t_k + \Delta t)] = [q]_{k+1} = [q]_k + [\dot{q}]_k \Delta t + \frac{1}{2}[\ddot{q}]_k \Delta t^2 + o(\Delta t^2)$$

$$[q(t_k - \Delta t)] = [q]_{k-1} = [q]_k - [\dot{q}]_k \Delta t + \frac{1}{2}[\ddot{q}]_k \Delta t^2 - o(\Delta t^2)$$

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- The finite differences are then:

$$[\dot{q}]_k^h = \frac{[q]_{k+1} - [q]_k}{\Delta t} = [\dot{q}]_k + \frac{1}{2}[\ddot{q}]_k \Delta t + o(\Delta t)$$

$$[\dot{q}]_k^{-h} = \frac{[q]_k - [q]_{k-1}}{\Delta t} = [\dot{q}]_k - \frac{1}{2}[\ddot{q}]_k \Delta t + o(\Delta t)$$

- And the central difference:

$$[\dot{q}]_k^{oh} = \frac{[q]_{k+1} - [q]_{k-1}}{2\Delta t} = [\dot{q}]_k + o(\Delta t)$$

- In first order approximation:

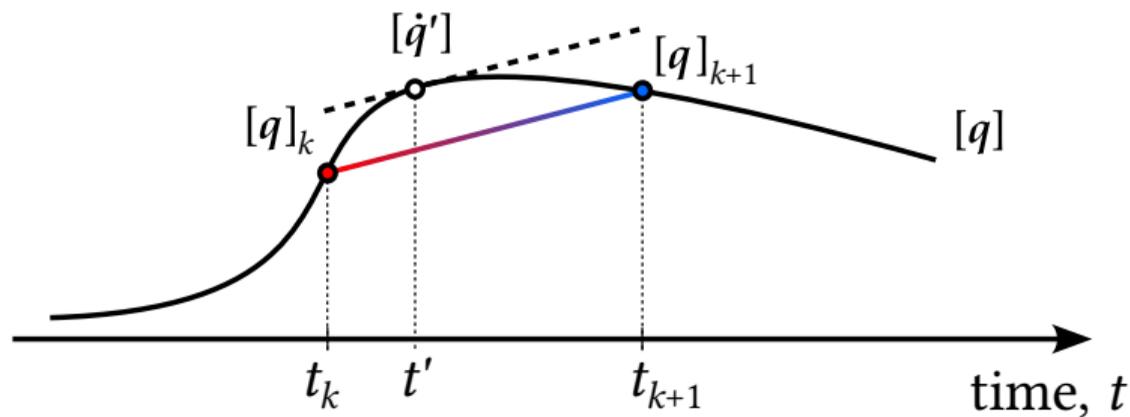
$$[\dot{q}]_k = \frac{[q]_{k+1} - [q]_k}{\Delta t} + O(\Delta t)$$

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- Note that notation $o(\Delta t)$ was changed to $O(\Delta t)$, where $y = O(x)$ means that $0 < \lim_{x \rightarrow 0} |y/x| < \infty$.

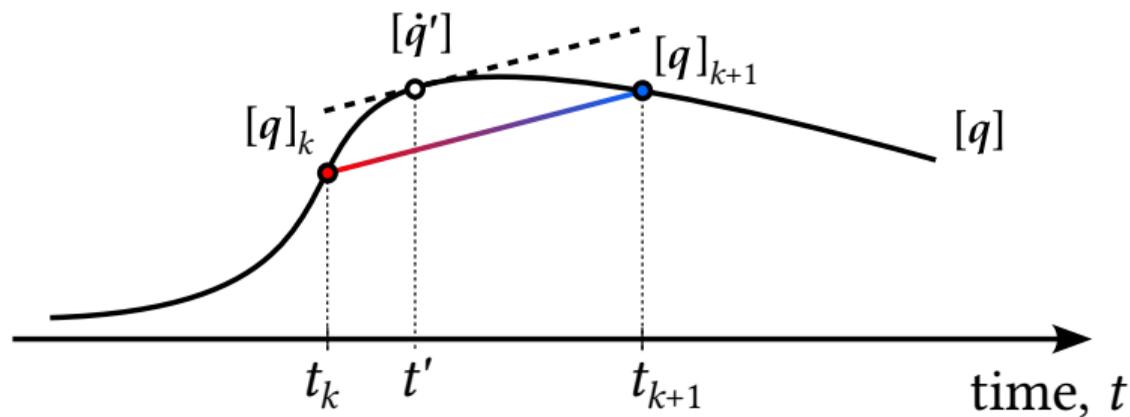
Mean value theorem



Th: If $[q] \in C^1([t_k, t_{k+1}])$ then $\exists t' \in [t_k, t_{k+1}]$ such that

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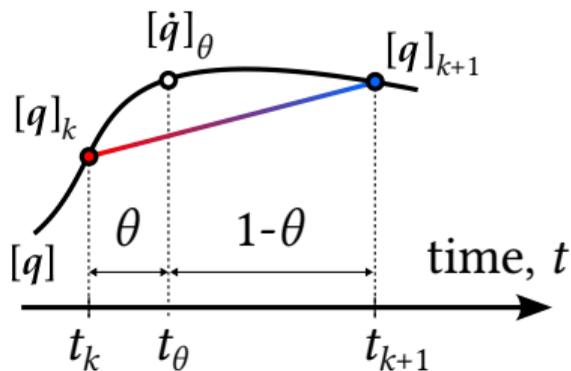
$$[q]_{k+1} - [q]_k = [\dot{q}(t')](t_{k+1} - t_k) \quad \Leftrightarrow \quad \frac{[q]_{k+1} - [q]_k}{\Delta t} = [\dot{q}(t')]$$

NB: Théorème des accroissements finis, Théorème de Lagrange

Integration methods

■ First order ODE:

$$[\dot{q}] = f([q]; t), \quad t \in \mathcal{T} = [0, T] \subset \mathbb{R}, \quad [q] \in \mathbb{R}^n$$



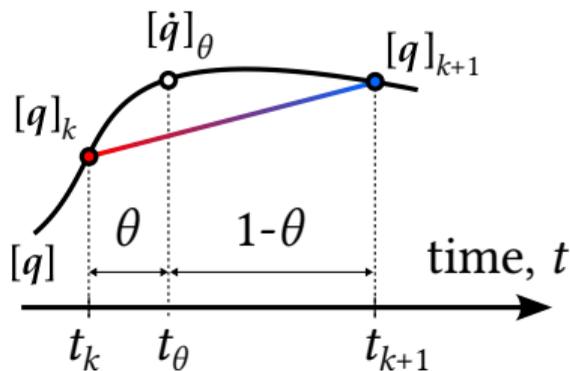
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$$t_k, t_{k+1} : \Rightarrow t_\theta = (1 - \theta)t_k + \theta t_{k+1} = t_k + \theta \Delta t$$



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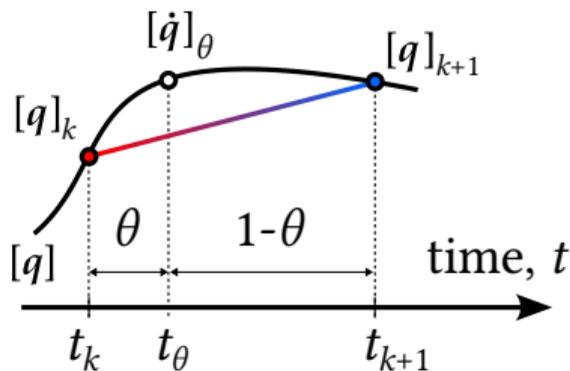
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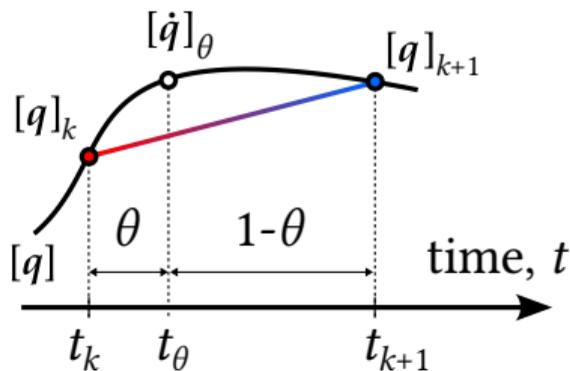
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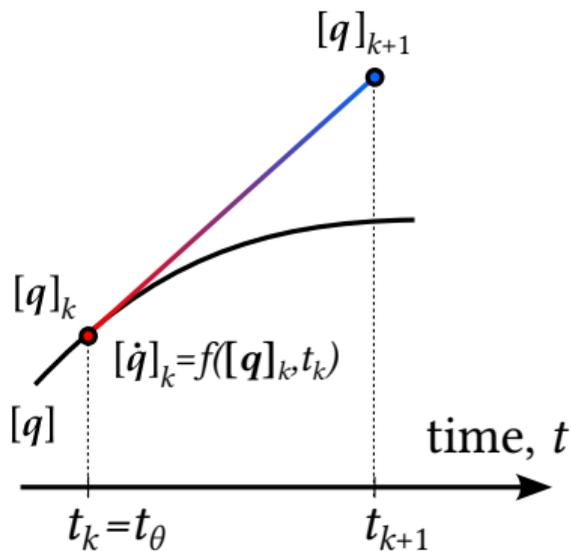


Methods:

- $\theta = 0$: Explicit (forward) Euler
- $\theta = 1$: Implicit (backward) Euler
- ★ $\theta = 0.5$: Crank-Nicolson method

Explicit integration

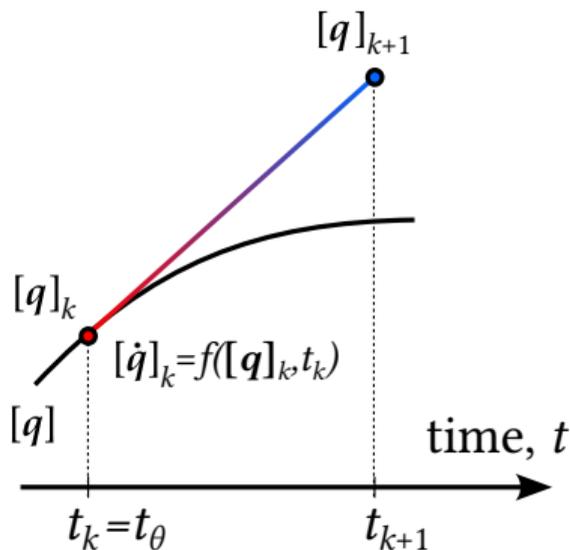
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$$\frac{[q]_{k+1} - [q]_k}{\Delta t} = f([q](t_k); t_k) + O(\Delta t)$$



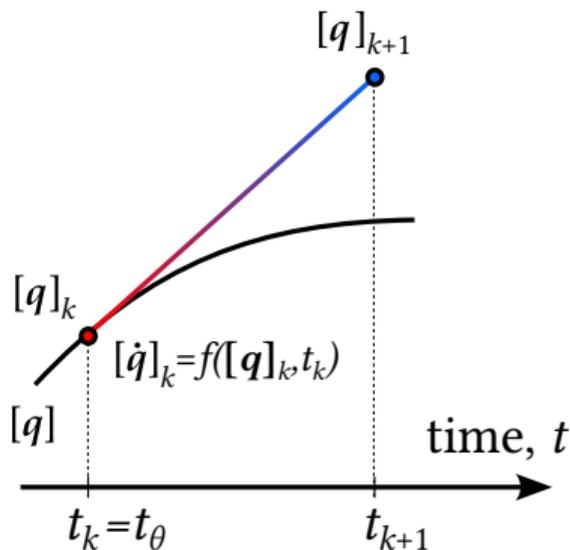
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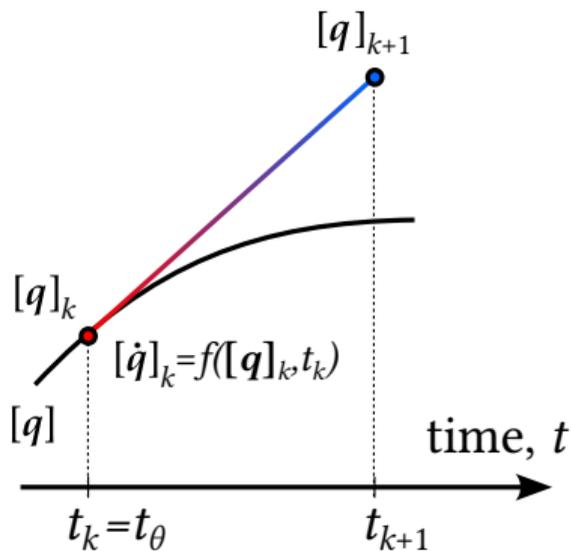
$$[q]_{k+1} = [q]_k + \Delta t f([q](t_k); t_k) + o(\Delta t)$$



Explicit integration for system of equations

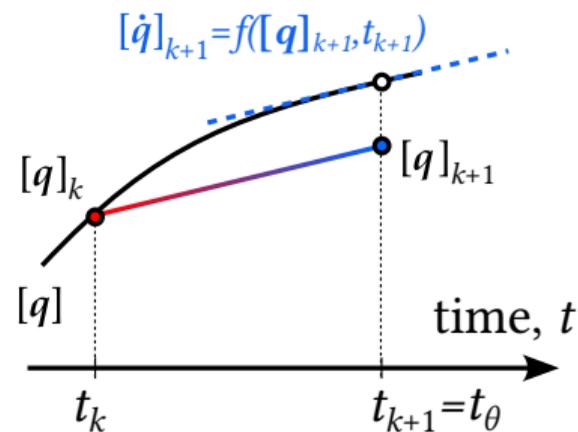
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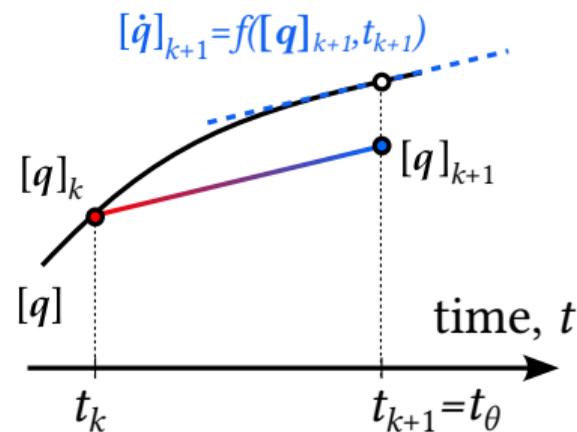
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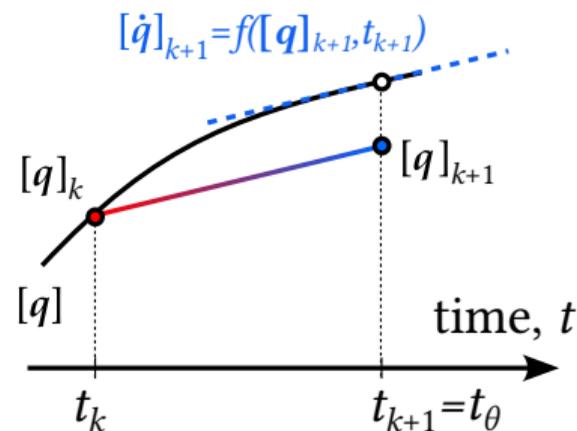
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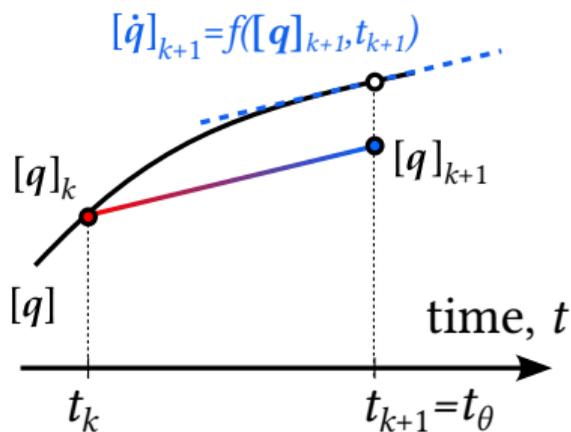
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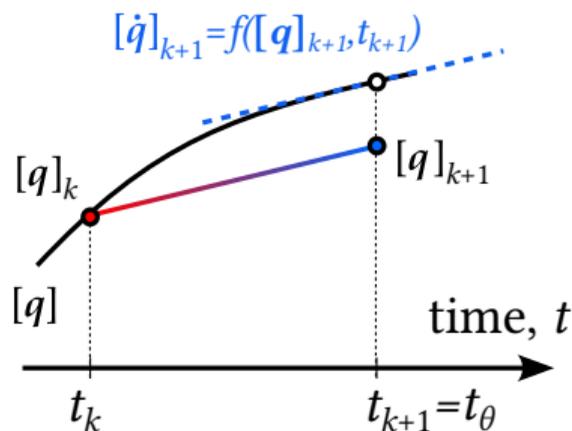
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- For system of equations:

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- Finite difference:

$$[C]([\mathbf{q}]_{k+1} - [\mathbf{q}]_k) = \Delta t ([F(t_{k+1})] - [K][\mathbf{q}]_{k+1}) + o(\Delta t)$$



Implicit integration for system of equations

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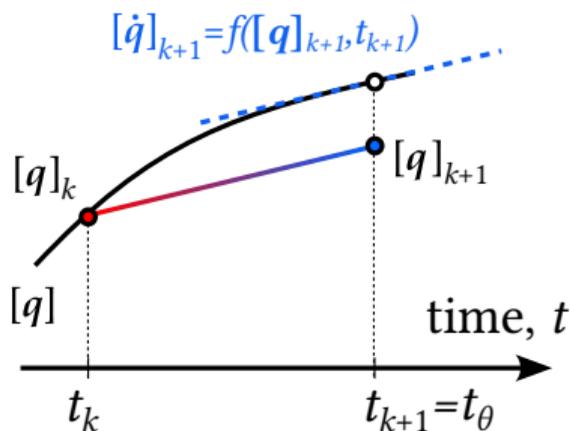
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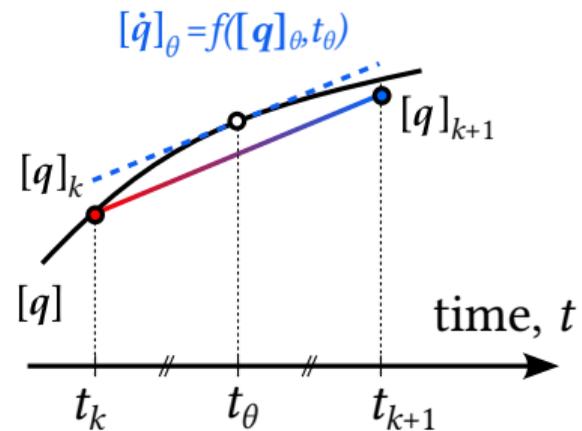
- Linear system of equations to be solved:

$$([C] + \Delta t[K])[\mathbf{q}]_{k+1} = [C][\mathbf{q}]_k + \Delta t [F(t_{k+1})]$$



Crank-Nicolson integration

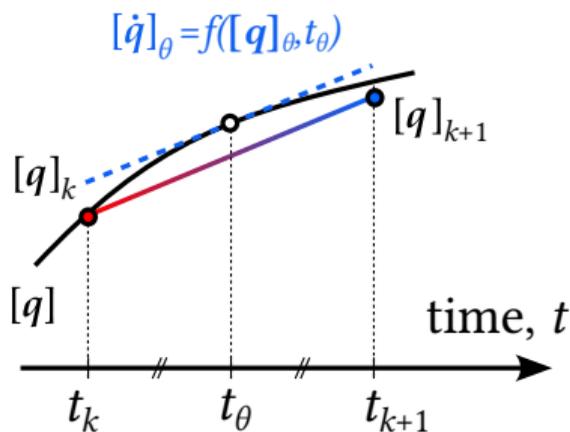
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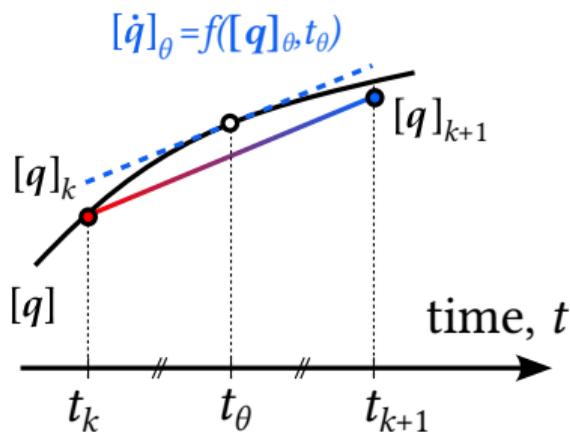
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$$f([q]_{k+1/2}; t_{k+1/2}) \approx \frac{1}{2} \left(f([q]_{k+1}; t_{k+1}) + f([q]_k; t_k) \right)$$



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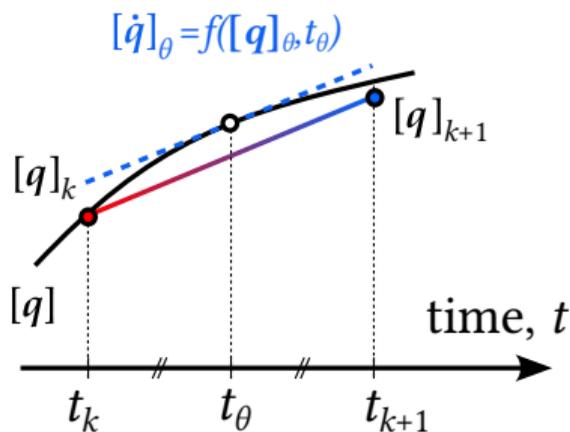
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$$f([q]_{k+1/2}; t_{k+1/2}) \approx \frac{1}{2} \left(f([q]_{k+1}; t_{k+1}) + f([q]_k; t_k) \right)$$

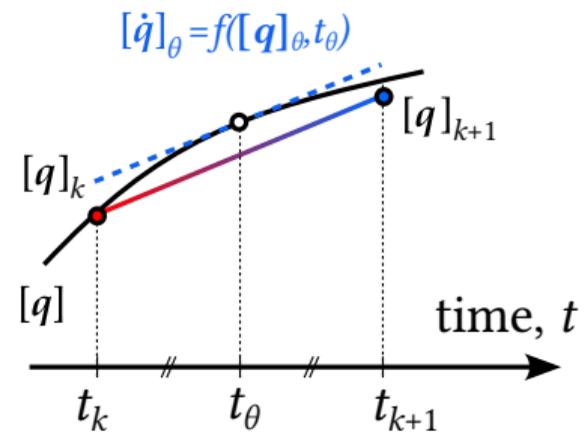
- Finally: $[q]_{k+1} = [q]_k + \frac{\Delta t}{2} \left(f([q]_{k+1}; t_{k+1}) + f([q]_k; t_k) \right) + o(\Delta t^2)$



Crank-Nicolson integration for system of equations

- For system of equations:

$$[C][\dot{q}] + [K][q] = [F(t)]$$



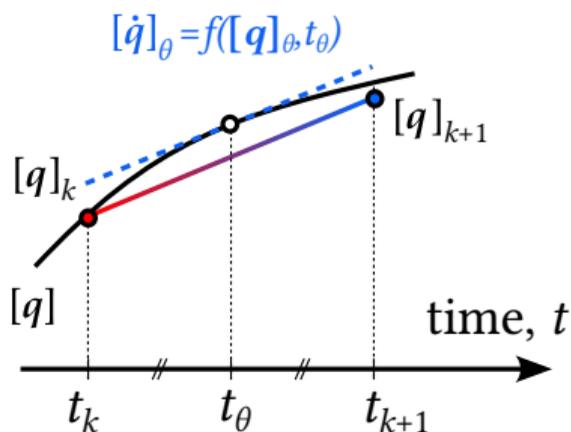
Crank-Nicolson integration for system of equations

- For system of equations:

$$[C][\dot{q}] + [K][q] = [F(t)]$$

- Finite difference:

$$[C]([q]_{k+1} - [q]_k) = \frac{\Delta t}{2}([F]_{k+1} + [F]_k - [K]([q]_{k+1} + [q]_k)) + o(\Delta t^2)$$



Crank-Nicolson integration for system of equations

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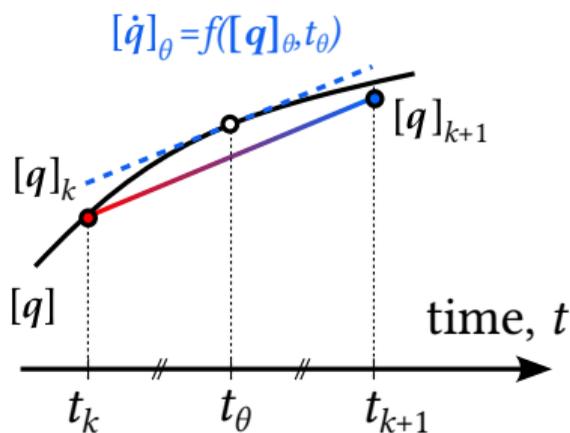
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- Linear system of equations to be solved:

$$\left([C] + \frac{\Delta t}{2}[K]\right)[q]_{k+1} = \left([C] - \frac{\Delta t}{2}[K]\right)[q]_k + \frac{\Delta t}{2}([F]_k + [F]_{k+1})$$



Example: 1D heat equation

- PDE

$$\dot{T}(x, t) = \alpha \Delta T(x, t), \quad x \in [0, 2], \quad t \in [0, \infty)$$

- Initial conditions

$$T(x, 0) = 0$$

- Boundary conditions:

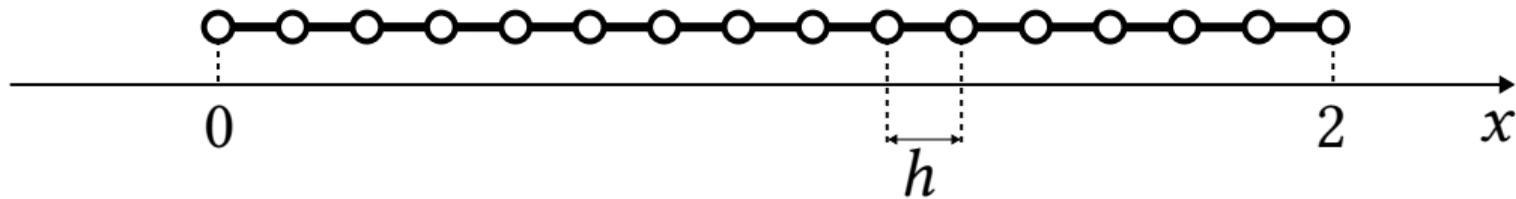
- Left edge $x = 0$: increase temperature $T(0, t) = T_0 t / t_0$

- Right edge $x = 2$: zero flux $q = \frac{\partial T}{\partial x} \Big|_{(2, t)} = 0$

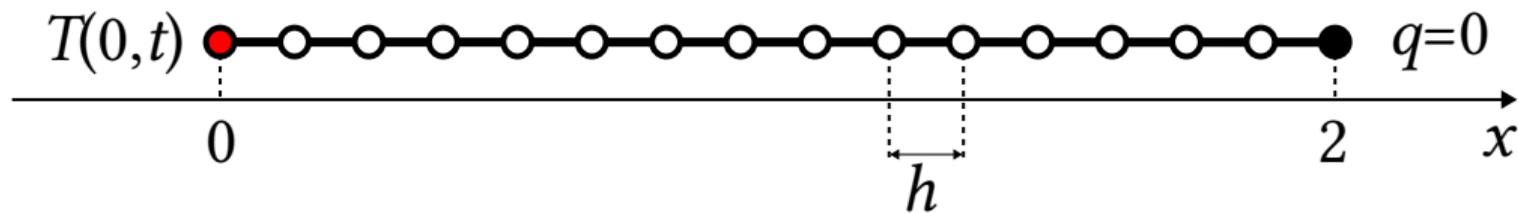
- Mesh: $N_x = 40, h = 0.05$ (l.u.)

- Parameter: $\alpha = 0.01$ (l.u.²/t.u.)

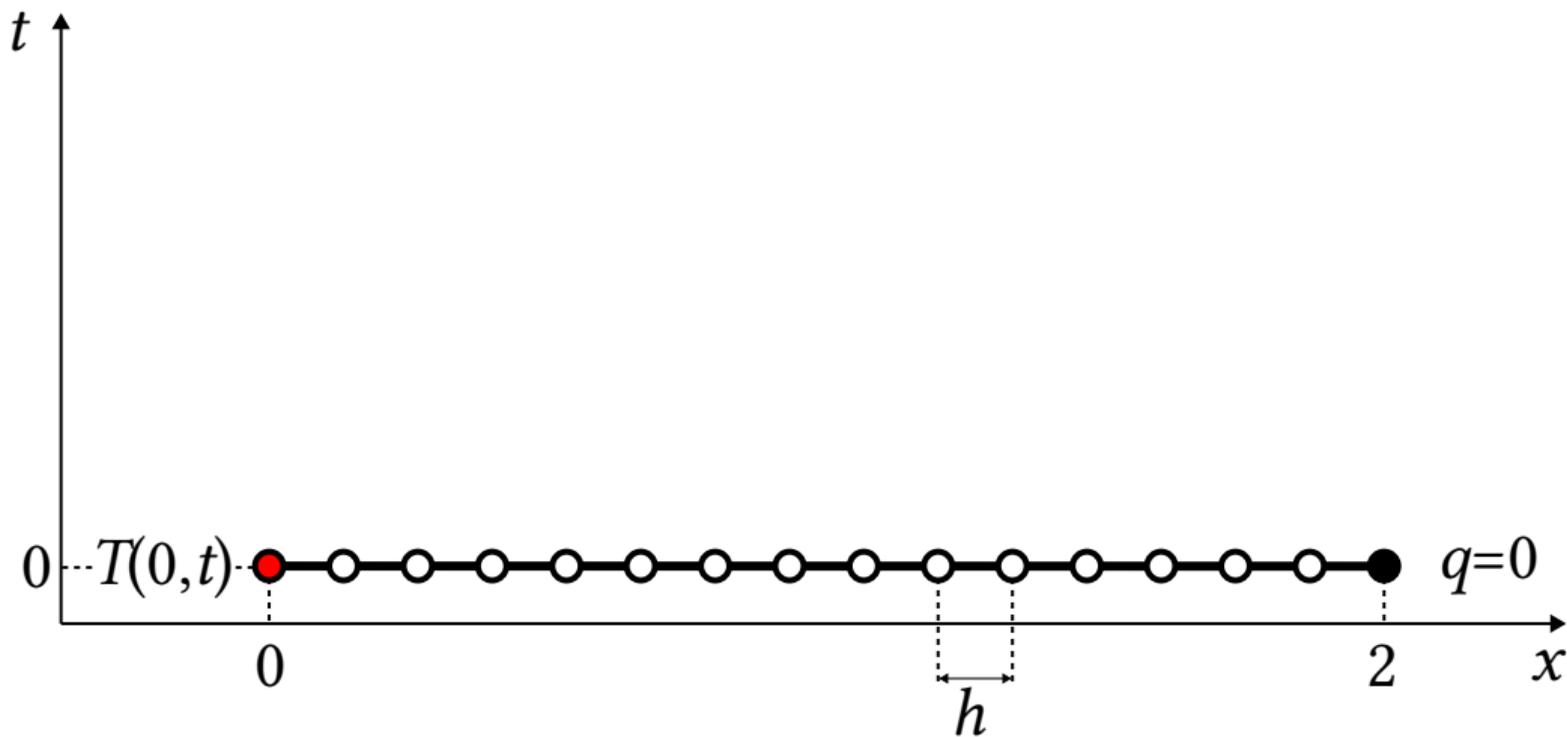
Example: 1D heat equation



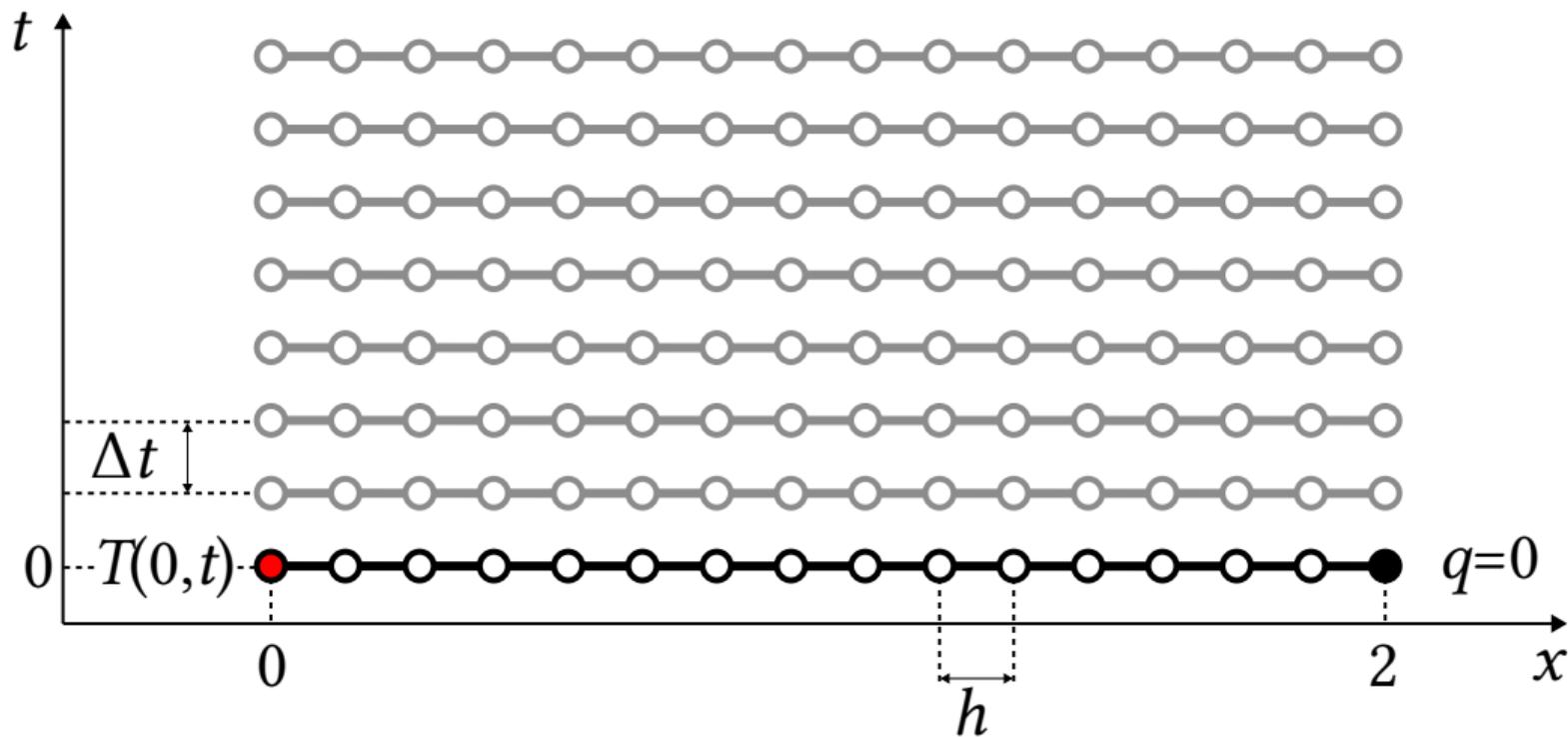
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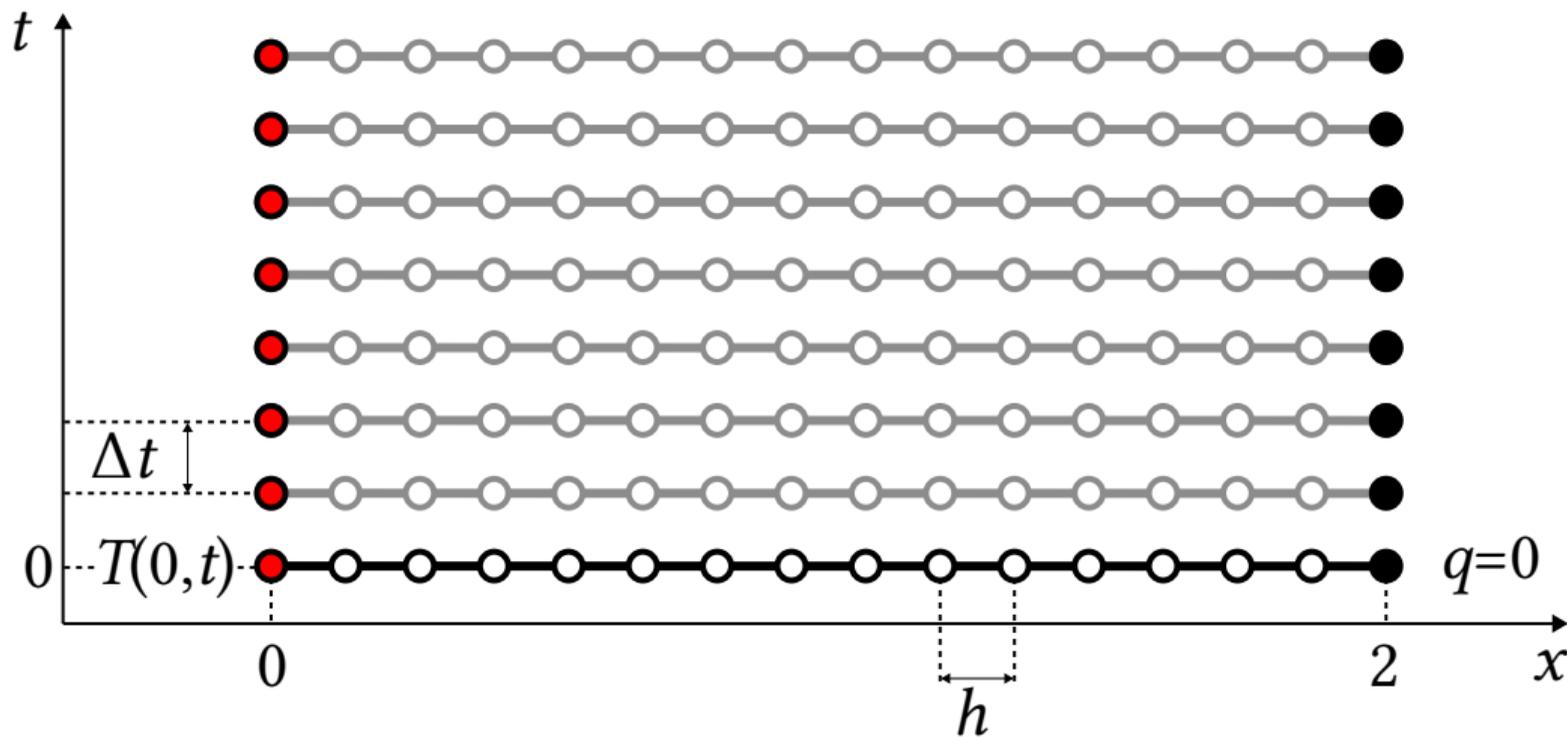
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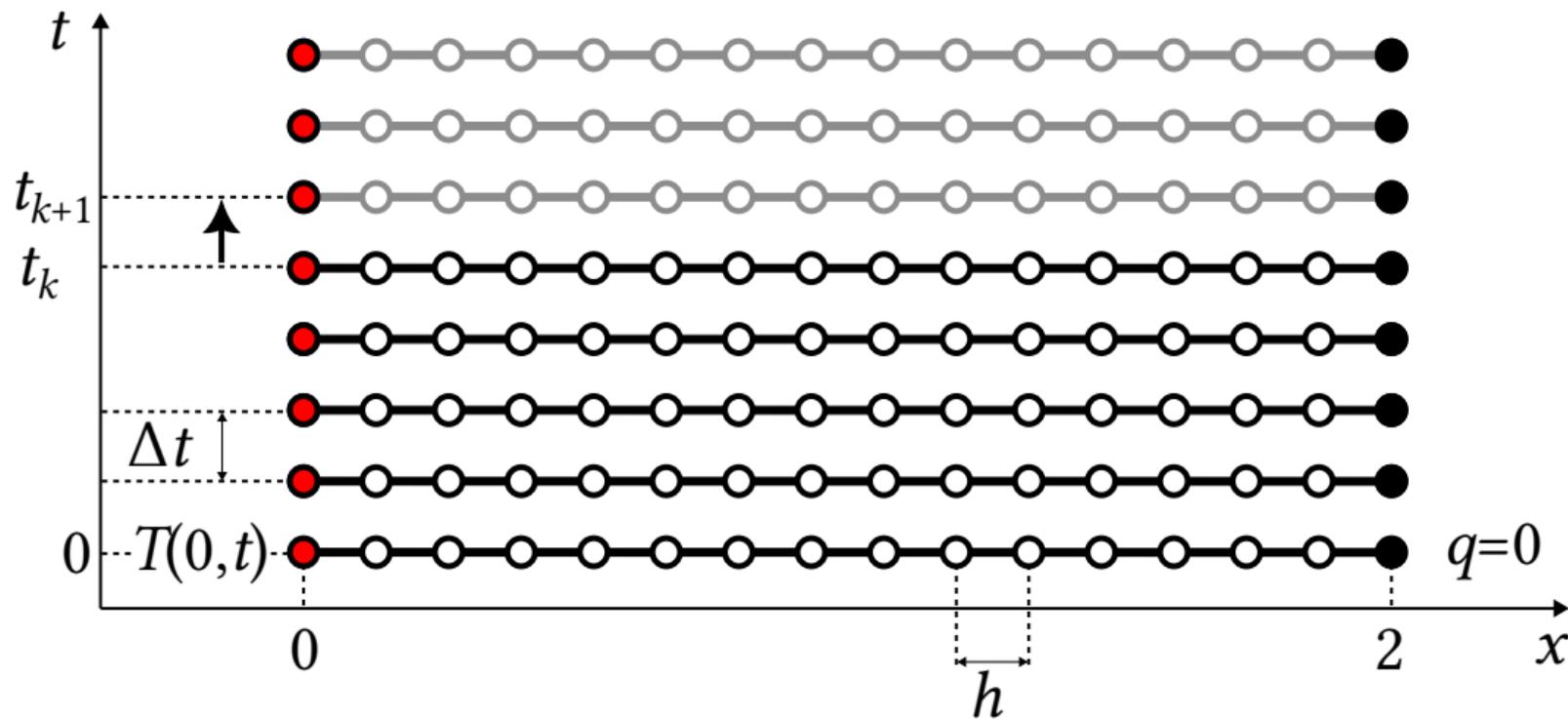
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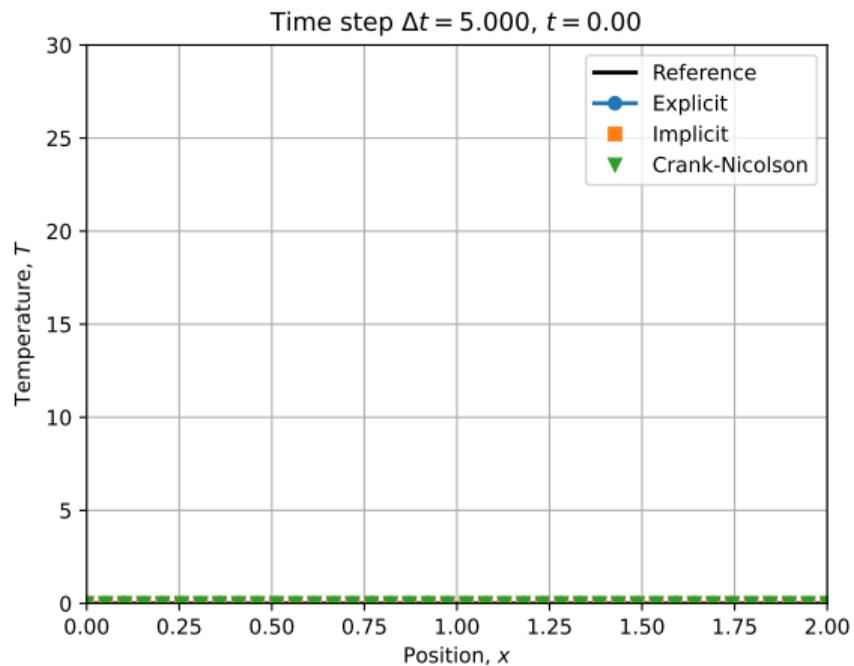
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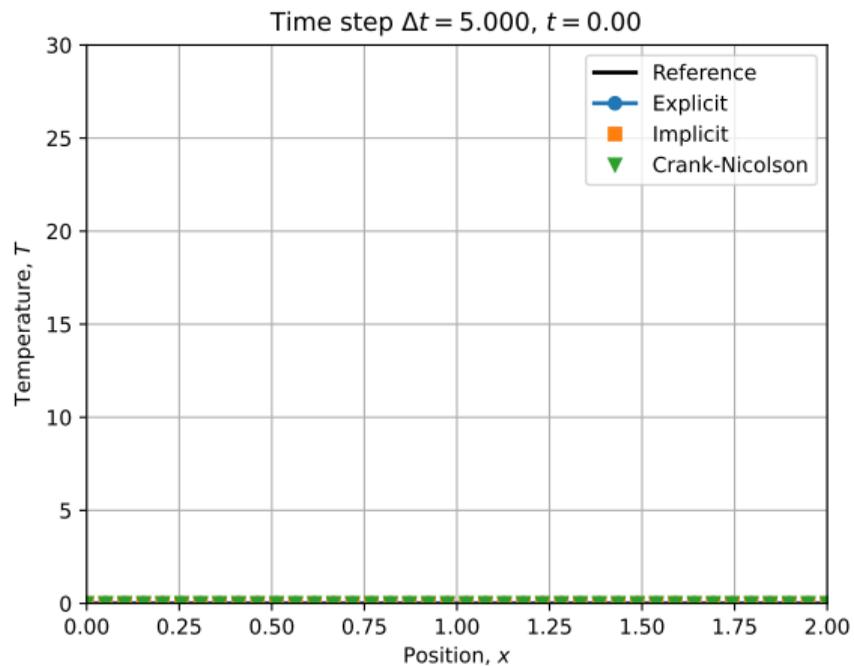


Example: integration results



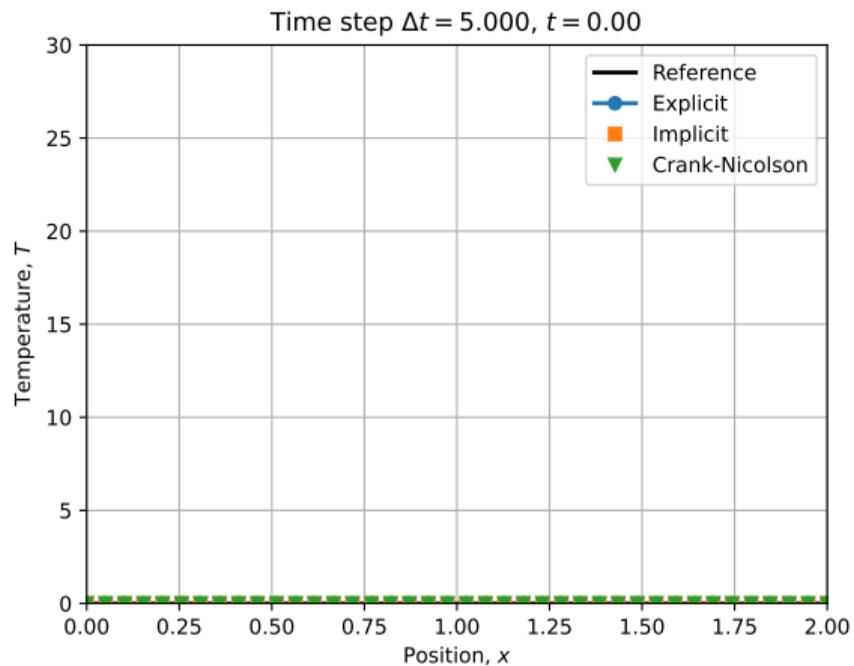
$$\Delta t = 0.10 \text{ t.u.}$$

Example: integration results



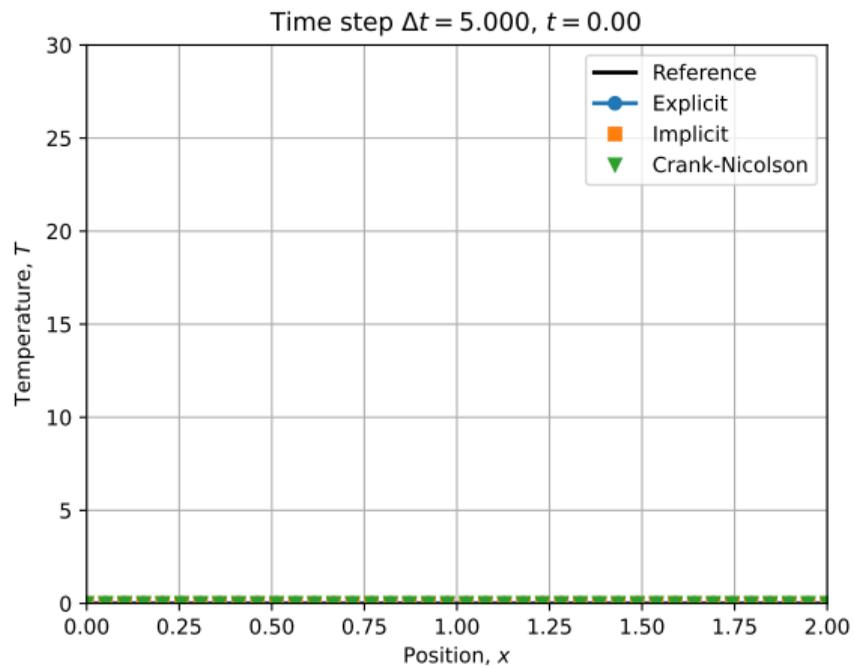
$$\Delta t = 0.13 \text{ t.u.}$$

Example: integration results



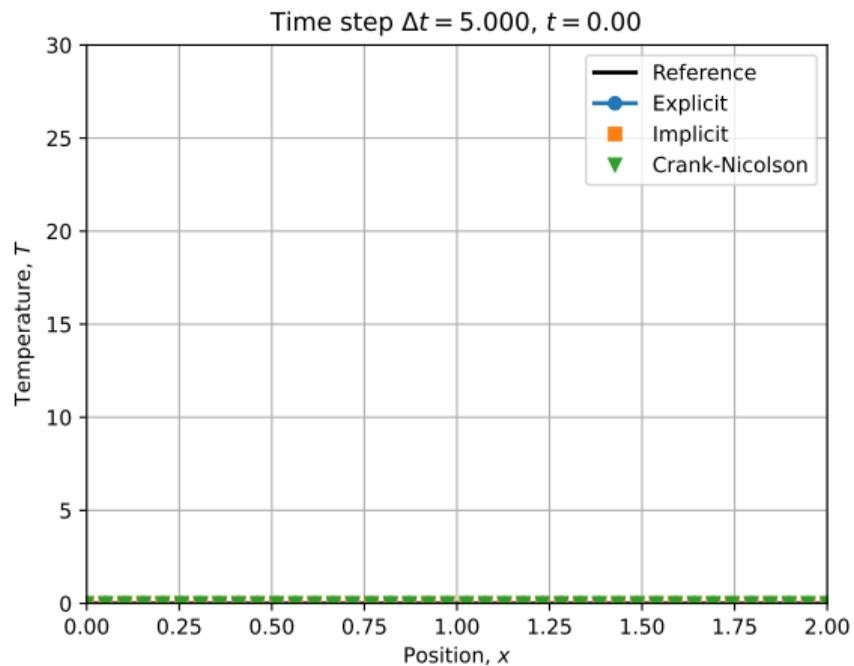
$\Delta t = 1.00$ t.u.

Example: integration results



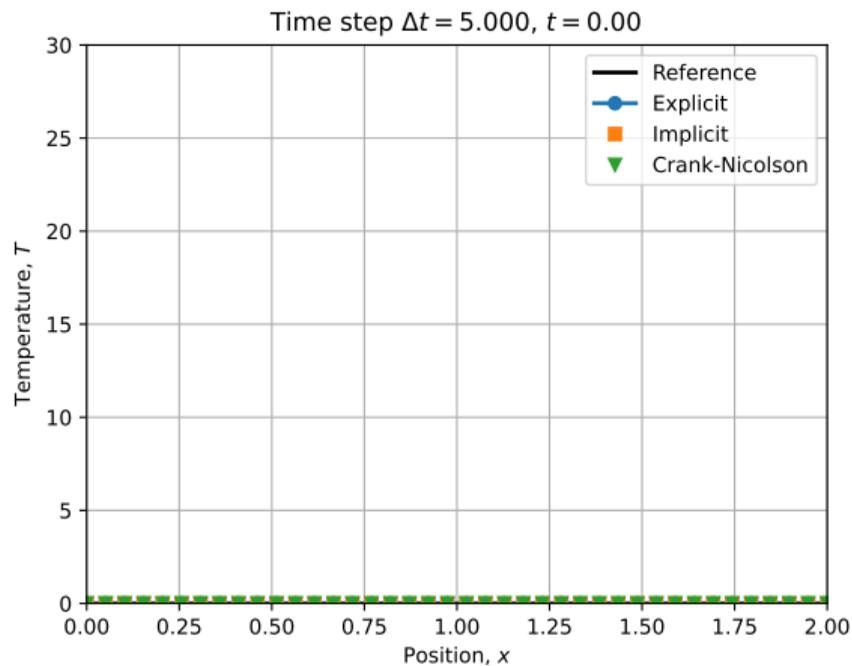
$$\Delta t = 2.00 \text{ t.u.}$$

Example: integration results



$\Delta t = 5.00$ t.u.

Example: integration results



$$\Delta t = 10.00 \text{ t.u.}$$

- For $\theta \geq 1/2$ the integration is unconditionally stable

[1] Courant, R.; Friedrichs, K.; Lewy, H. (1928), Über die partiellen Differenzgleichungen der mathematischen Physik (in German), *Mathematische Annalen* 100 (1): 32-74

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NB: Richard Courant was a doctoral student and assistant of David Hilbert.

Stability criterion

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- Courant-Friedrichs-Lewy^[1,2] or CFL condition
the signal should not propagate more than one element in one time step:

for $\theta < 1/2$: for stability $\Delta t_c = Ch^2$

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- The smallest element of the mesh will control the critical time step
one more reason to be careful with your mesh (or with your integrator)

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Second order differential equations

Solid dynamics: explicit integrators

- Discretized equations:

$$[M][\ddot{u}] + [C][\dot{u}] + [K][u] = [F](t)$$

with mass matrix $[M] \in \mathbb{R}^{n \times n}$,
viscous damping matrix $[C] \in \mathbb{R}^{n \times n}$,
stiffness matrix $[K] \in \mathbb{R}^{n \times n}$,
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- For explicit integrators a similar CFL condition exist: the signal propagating at speed $c_l = \sqrt{E/\rho}$ should not propagate more than the smallest element $\min\{h\}$, resulting in

$$\Delta t < \Delta t_c = \min\{h\} \sqrt{\frac{\rho}{E}}$$

- For damping matrix $[C]$, Rayleigh damping is often employed:

$$[C] = \mu[M] + \lambda[K]$$

so the damping is frequency dependent in the following way

$$\text{Amplitude} \sim \exp(-\xi t) : \quad \xi(\omega) = \frac{1}{2} \left(\frac{\mu}{\omega} + \lambda\omega \right)$$

Solid dynamics: implicit integrators

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- So implicit (unconditionally stable) integrators are of interest

Solid dynamics: implicit integrators

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- Quite often only “low mode” response is of interest
- So implicit (unconditionally stable) integrators are of interest
- Need to control the dissipation of high modes with a parameter other than time step.
- This dissipation should not strongly affect lower modes.

Hilber-Hughes-Taylor implicit integrator^[1]

- Discretized equations and initial conditions:

$$[M][\ddot{u}] + [K][u] = [F](t), \quad [u]_0 = [u_0], \quad [\dot{u}]_0 = [v_0]$$

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- Integrator with three parameters α, β, γ :

$$\begin{aligned} [M][\ddot{\mathbf{u}}]_{k+1} + (1 + \alpha)[K][\mathbf{u}]_{k+1} - \alpha[K][\mathbf{u}]_k &= [F]_{k+1} \\ [\mathbf{u}]_{k+1} &= [\mathbf{u}]_k + \Delta t[\dot{\mathbf{u}}]_k + \Delta t^2 \left[(1/2 - \beta)\ddot{\mathbf{u}}_k + \beta\ddot{\mathbf{u}}_{k+1} \right] \\ [\dot{\mathbf{u}}]_{k+1} &= [\dot{\mathbf{u}}]_k + \Delta t \left[(1 - \gamma)\ddot{\mathbf{u}}_k + \gamma\ddot{\mathbf{u}}_{k+1} \right] \end{aligned}$$

- Where initial accelerations are initiated as

$$[\ddot{\mathbf{u}}]_0 = [M]^{-1} ([F]_0 - [K][\mathbf{u}]_0)$$

[1] Hilber, H.M., Hughes, T.J.R. and Taylor, R.L. (1977) "Improved Numerical Dissipation for Time Integration Algorithms in Structural Dynamics", Earthquake Engineering and Structural Dynamics 5:283-292

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- Spectral radius

$$\rho = \max_i \{\lambda_i\}$$

- By repetitive use of $[\mathbf{X}]_{n+1} = [\mathbf{A}][\mathbf{X}]_n$ and eliminating $\Delta t\dot{u}, \Delta t^2\ddot{u}$

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- Explicit form of the amplification matrix:

$$[A] = \frac{1}{D} \begin{bmatrix} 1 + \alpha\beta\Omega^2 & 1 & 1/2 - \beta \\ -\gamma\Omega^2 & 1 - (1 + \alpha)(\gamma - \beta)\Omega^2 & 1 - \gamma - (1 + \alpha)(1/2\gamma - \beta)\Omega^2 \\ -\Omega^2 & -(1 + \alpha)\Omega^2 & -(1 + \alpha)(1/2 - \beta)\Omega^2 \end{bmatrix}$$

where

$$D = 1 + (1 + \alpha)\beta\Omega^2$$

$$\Omega = \omega\Delta t$$

$$\omega = \sqrt{K/M}$$

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$$\begin{cases} A_1 = 1 - \Omega^2/(2D) + A_3/2 \\ A_2 = 1 + 2A_3 \\ A_3 = \alpha(1 + \alpha)^2\Omega^2/(4D) \end{cases}$$

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where D becomes $D = 1 + (1 + \alpha)(1 - \alpha)^2\Omega^2/4$

- So eigenvalues could be found from:

$$(\lambda - A_3)(\lambda - 1)^2 + \Omega^2\lambda^2/D = 0$$

- In the limit $\Omega \rightarrow \infty$

$$\left[(1 - \alpha)(1 - \alpha)^2 \lambda - \alpha(1 + \alpha)^2 \right] (\lambda - 1)^2 + 4\lambda^2 = 0$$

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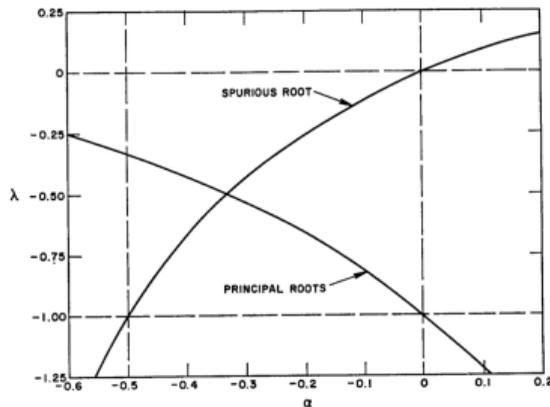


Figure 1. Eigenvalues of the amplification matrix in the limit $\Delta t/T \rightarrow \infty$ vs α

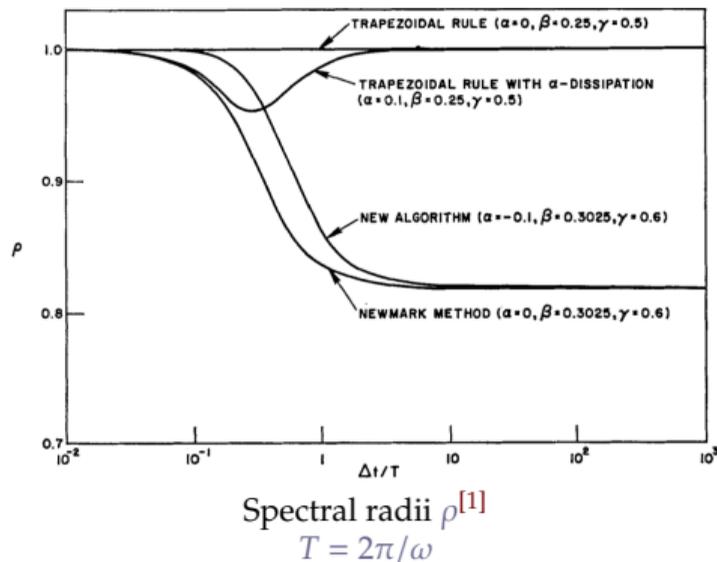
Figure from^[1]

\Rightarrow HHT integrator is stable if $-1/2 \leq \alpha \leq 0$

[1] Hilber, H.M., Hughes, T.J.R. and Taylor, R.L. (1977) "Improved Numerical Dissipation for Time Integration Algorithms in Structural Dynamics", Earthquake Engineering and Structural Dynamics 5:283-292

Comparison

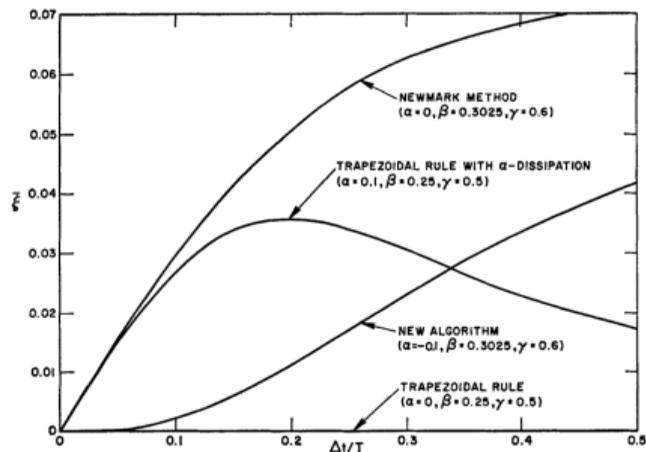
- (1) Trapezoidal rule $\alpha = 0, \beta = 0.25, \gamma = 0.5$
- (2) Trapezoidal rule with damping $\alpha = 0.1, \beta = 0.25, \gamma = 0.5$
- (3) Newmark with γ damping $\alpha = 0, \beta = 0.3025, \gamma = 0.6$
- (4) HHT $\alpha = -0.1, \beta = 0.3025, \gamma = 0.6$



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Damping factor^[1] $\xi : u_n \sim \exp(-\xi \omega t_n)$
 $T = 2\pi/\omega$

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Examples

Merci de votre attention !